

CITY OF NAPERVILLE
DESIGN MANUAL FOR PUBLIC IMPROVEMENTS
JANUARY 2022 UPDATE

SECTION 1: COMMERCIAL SITE DESIGN

SECTION 2: STORMWATER MANAGEMENT

SECTION 3: SANITARY SEWER DESIGN

SECTION 4: STREET DESIGN

SECTION 5: WATER DISTRIBUTION SYSTEM DESIGN

The following design elements and considerations are contained in the City of Naperville's Municipal Code. For these items, please reference the code chapters and sections identified below through the City's website for the current standards and requirements.

SECTION 6-9-3: SCHEDULE OF OFF-STREET PARKING REQUIREMENTS

CHAPTER 14: EXTERIOR LIGHTING STANDARDS

CHAPTER 10: LANDSCAPING, SCREENING AND TREE PRESERVATION

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SECTION 1

COMMERCIAL SITE DESIGN

1.1 GENERAL DESIGN CRITERIA

This section covers the City of Naperville standards for site engineering for non-residential developments and apartments. Design elements that are incorporated into this section include parking areas, vehicular and pedestrian access, and drainage. Stormwater detention requirements are provided in Section 2 of this manual.

The City of Naperville has adopted the standards of the Institute of Transportation Engineers (ITE) for parking lot design. Reference is made to the Institute of Transportation Engineers (ITE) publication *Guidelines for Parking Facility Location and Design*, latest edition.

1.2 SITE ACCESS

1.2.1 Commercial Driveway Geometrics

Commercial driveways designed for one-way traffic flow shall not exceed 20 feet in width measured at the property line. Commercial driveways designed for two-way traffic flow shall have a minimum width of 24 feet and shall not exceed 35 feet in width measured at the property line. This width dimension is measured between the face of curbs.

The minimum turning radius for commercial driveways, measured along the curb return, shall be 15 feet. A right-in/right-out driveway access should have a 50-foot radius, measured along the curb return. Any deviation from this design shall be approved by the City Engineer.

The angle between the curb line of the street and the centerline of a full access driveway shall not be less than 60 degrees as measured from the back of curb.

1.2.2 Commercial Driveway Safety Standards

No commercial driveways will be permitted into any parking lot or other facility which is designed in such a way as to make it necessary for exiting vehicles to back onto the street.

No driveway will be permitted into any facility which would require and/or allow a vehicle to drive or maneuver on the sidewalk area in any manner other than to cross it.

No driveway will be permitted for the purpose of allowing vehicles to park on the public right-of-way.

In no case shall a driveway be constructed in such a way as to present a hazard to pedestrians or traffic on the public right-of-way.

In no case shall an object located within the right-of-way be permitted to obscure the vision of drivers of motor vehicles. At minimum, items in the right-of-way within the required 30-foot sight triangle, shall be limited in height to no greater than 30 inches, and no less than 6 feet, including shrubs and tree branches.

1.2.3 Commercial Driveways on Arterial Streets

Any driveway onto an arterial street represents a potential impediment to traffic and/or a safety hazard. For this reason, access onto arterial streets shall be limited both in number of driveways and location and may be granted only after review of the overall land development plan and traffic study for the project. Design criteria as established in this section represent minimum standards. Where hardships are demonstrated, exceptions to the minimum standard will be considered on a project-by-project basis. The use of cross-access easements or frontage roads are preferable alternatives to additional driveways on arterial streets.

1.2.3.1 Minimum Spacing

Driveways shall be located as far apart as practical. A minimum of 400 feet between centerlines of driveways on arterial streets (major and minor) should be sought.

1.2.3.2 Minimum Distance from Intersections

Driveway placement should always be designed to maximize the distance from the nearest intersections. Along major and minor arterial streets, the minimum distance of a full-access driveway from an intersecting public street shall be 250 feet. This distance is measured from the near edge of the intersection to the centerline of the driveway. Right-in/right-out driveways may be located closer, provided that they do not cause operational problems at the intersection.

1.2.4 Commercial Driveways on Collector and Local Streets

1.2.4.1 Location

Driveways on collector and local streets shall be located in accordance with the following:

The distance from the end of the driveway curb cut to the prolongation of the nearest intersecting street property line shall not be less than 20 feet on the near side of the intersection and not less than 10 feet on the far side.

The distance from the end of the driveway curb cut to the end of the intersecting street curb return shall not be less than 5 feet.

The distance from the end of the driveway curb cut to the nearest crosswalk shall not be less than 5 feet.

The distance between driveways, measured at the curb line of the street (from the end of the curb cut to the end of the other curb cut), shall not be less than 20 feet.

Where bus stops exist at locations where driveways are desired, the minimum allowable distance between driveways, measured at curb line of the street (from the end of the curb cut to the end of the other curb cut), shall be 40 feet.

No driveway shall be constructed which enters a public street within the limits of an intersection, with the limits of the intersection being defined as the area included within the prolongation of the lateral boundary lines of two or more streets or highways which join one another at an angle whether or not one such street or highway crosses the other.

At intersections where a separate right-turn lane exists, no driveway shall be constructed where the edge of the turning lane taper pavement is greater than 5 feet from the edge of the through pavement.

1.3 PARKING LOTS

1.3.1 Required Number of Parking Spaces

Refer to Section 6-9-3 of the City code (see Appendix A).

1.3.2 Stall and Aisle Dimensions

Parking stall dimensions shall be 9 feet wide by 17.5 feet in length, or as otherwise demonstrated to be in accordance with Table 2 of the *Guidelines for Parking Facility Location and Design*, published by ITE.

Minimum drive aisle width for two-way traffic is 26 feet.

For multi-family / townhome developments the minimum distance between two parallel buildings with shared vehicular access is 61 feet to accommodate a drive aisle and perpendicular parking spaces on each side.

1.3.3 Parking Lot Island Requirements

Islands are required at the ends of all parking rows and at intermediate locations such that there is a maximum of twenty (20) stalls between islands. Landscape islands shall have a concrete curb and be a minimum of one hundred thirty (130) square feet in area and a minimum of eight (8) feet in width, as measured from back of curb to back of curb.

1.3.4 Parking Lot Pavement

All parking lots constructed in the City of Naperville must have a paved surface and a minimum pavement structural number of 2.5. The structural coefficients for pavement design are as follows:

STRUCTURAL MATERIAL	COEFFICIENT
Bituminous Surface	0.40
Bituminous Binder	0.33
Bituminous Base Course	0.30
Aggregate Base Course (CA-6)	0.13
Aggregate Sub-base (CA-6)	0.12

1.3.5 Boundary Controls

All parking lots shall have 6-inch concrete barrier curb around the perimeter.

1.3.6 Drainage

Storm sewers which serve parking lots shall be designed to accommodate the 10-year storm event without surcharging out of the rim. The maximum depth of ponding in parking lots is 1 foot. This applies where parking lots are used for stormwater detention and for other lots in the event that all storm sewers are blocked, and surface overflows must be used to drain the lot.

1.4 LIGHTING

The City has developed exterior lighting standards to control glare and obtrusive light while maintaining a safe and secure environment for pedestrians and property. These standards are incorporated into City Code Title 6 Chapter 14 (see Appendix B).

1.5 PEDESTRIAN ACCESS

All commercial sites shall be designed so that sidewalks or other delineated pedestrian routes are available to provide pedestrian access continuity between the public sidewalk adjacent to the site and the main entrance to the building.

1.6 LANDSCAPING

Parkway landscaping, perimeter landscaping, parking lot landscaping and refuse area screening shall be required as specified in Title 5 Chapter 10 of the City code, or as otherwise specified and approved in a required landscape plan (See Appendix C). No trees shall be placed within 10 feet of any storm structure or 5 feet of any storm line.

1.7 BARRIER-FREE ACCESSIBILITY

All commercial sites shall comply with the accessibility requirements of the Illinois Accessibility Code, as amended; the provisions of Title 5 (Building Regulations) of the City of Naperville Municipal Code, as amended; and the City of Naperville Fair Housing Ordinance, as amended.

To ensure compliance with the requirements referenced above, the following guidelines should be considered in the site design for new construction of commercial projects (and may not apply for additions, alterations, or historic preservation):

An accessible route shall be provided from accessible parking and passenger loading zones to an accessible entrance. Accessible routes shall be constructed with a maximum cross slope of 2.0% and be free from obstacles.

The cross slope of sidewalks should be kept at a maximum of 2.0%, but with a minimum of 0.5% as necessary for drainage. This will make it easier for a person utilizing a wheelchair to move forward without veering left or right.

Sidewalks should be flush with grass areas on either side to help prevent wheelchairs from overturning should a wheel roll off the sidewalk.

Sidewalks should be 5 feet wide, minimum, to allow two wheelchairs to pass. In high pedestrian traffic areas, such as the Central Business District, sidewalks should be 8 feet wide.

Where passenger loading zones are provided, an adjacent access aisle should be provided where the sidewalks are flush with the pavement.

Entrance areas near the door should meet ADA landing standards (allowing for proper drainage to avoid ponding and icing).

All power door pedestals with push plates should be clear of the door swing, typically 5 feet from the door.

Accessible entrances shall be considered for secondary entrance points in addition to the main entrance.

Accessible parking stalls should be close to both the main and auxiliary entrances, to provide maximum access for persons with disabilities.

Accessible parking stalls should be constructed with maximum slopes of 2.0% which provides a nearly flat surface for wheelchairs and minimum slopes necessary for drainage.

Concrete wheel stops are not allowed in the parking lots. Bollards can be used in lieu of wheel stops.

The sidewalk adjacent to accessible parking stalls should be flush with the pavement to provide an accessible route to an entrance.

Accessible parking spaces shall be appropriately designated through signage and striping. Signs shall be vertically mounted on a post or wall with sign placement to follow City Standard Detail 590.35, no more than 5 feet horizontally from the front of the parking space and set a minimum of 4 feet from finished grade to the bottom of the sign.

Speed bumps within parking lots are not recommended as they can create a hazard for low riding wheelchair vans.

SECTION 2 STORMWATER MANAGEMENT

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SECTION 2 STORMWATER MANAGEMENT

2.1 ADOPTION OF BEST MANAGEMENT PRACTICES

The City of Naperville has adopted the Best Management Practices (BMP) approach for the design of all stormwater and floodplain management system components. The basis of this approach is the City's support and membership in the DuPage River Salt Creek Workgroup and The Lower DuPage River Watershed Coalition.

Except as modified and/or appended herein, all stormwater management components covered in this section shall be designed as Best Management Practices (BMP's) in accordance with the most current edition of "Urban Stormwater Best Management Practices for Northeastern Illinois" published by the Northeastern Illinois Planning Commission.

2.2 HYDROLOGIC ANALYSIS

2.2.1 General

The City of Naperville uses the term "Design Storm" to define precipitation events used in the design and analysis of all stormwater management facilities in the City. The Design Storm is the frequency with which a given rainfall event is equaled or exceeded, on average, once in a period of years. The probability of occurrence is equal to the reciprocal of the design storm frequency. For example, a 100-year design storm has a 1.0 percent probability of occurrence in any one year.

2.2.2 Rainfall Data

The Illinois State Water Survey (ISWS) publication entitled "Precipitation Frequency Study for Illinois" commonly called ISWS Bulletin 75 shall be used as the rainfall data reference source for the design of stormwater management facilities.

2.2.3 Design Storm

The standard design storm for all Minor Drainage System components will be the appropriate "10-year recurrence interval" rainfall, as defined in ISWS Bulletin 75. The standard design storm for all Major Drainage System components will be the appropriate "100-year recurrence interval" rainfall, as defined in ISWS Bulletin 75.

2.2.4 Topographic Data

Topographic data shall be referenced to at least one established City of Naperville benchmark. Contour intervals shall be one foot and shall directly correlate to the City datum.

2.2.5 Rainfall-Runoff Modeling Methodology

The following summarizes the application of common rainfall-runoff models:

Model	Product	Usage	Assumptions	Limitations
Rational Method	Peak Discharge	For estimating peak discharge rate.	Watershed size 1-20 acres	
TR-20/WIN TR-20	Hydrograph	For developing: Runoff hydrographs, Channel and reservoir routed hydrographs, Separated or combined hydrographs.		
TR-55 Tabular Method	Hydrograph	For measuring the effect of land use and/or structure changes in sub-catchments on the composite hydrograph for the watershed	<ul style="list-style-type: none"> • Sub-catchment size less than 20 square miles. • Precipitation runoff volume not less than 1.5 inches. • Little to no variation in soil and land use characteristics within a sub-catchment. 	The TR-55 Tabular Method was derived from TR-20 using curve numbers of 75 and rainfall amounts sufficient to produce 3 inches of runoff volume. Characteristics should not vary significantly from those used in the method derivation.
TR-55 Graphical Method	Peak Discharge	For estimating peak discharge rate.	Same as the TR-55 Tabular Method. In addition, the initial abstraction must be less than 25 percent of the total 24-hour rainfall volume.	The Graphical Method was developed from the TR-55 Tabular Method; therefore, the limitations are the same.
TR-55 Chart Method	Peak Discharge	For estimating the effect of development on peak discharge rate.	Watershed size: 1-2000 acres	
HEC-1/HEC-HMS	Hydrograph	For computation of flood hydrographs due to a single recorded or synthetic rainstorms, or snowfall-snowmelt conditions. Model can simulate dam-break conditions and compute expected annual flood damages.	Hydrologic processes can be simulated assuming that individual parameters can be used in representing average sub-basin temporal and special characteristics.	<ul style="list-style-type: none"> • Only one storm can be modeled for each program run because the model does not have a provision for soil moisture recovery during non-precipitation periods. • Results are in terms of discharge, not stage. If discharges are stage-dependent, a user specified rating curve must be provided. • Hydrologic routings used for open channel flow. If gradually varied, unsteady flow occurs, such as in very flat channels, then hydrologic routings are not accurate. • Hydrologic routings used for storage reservoirs. If reservoir outflow is dependent on downstream controls, then hydrologic routings are not accurate.

2.3 HYDRAULIC ANALYSIS

2.3.1 General

The following table summarizes the application of common hydraulic models:

Model	Usage	Assumptions	Limitations
HEC-2	For computing water surface profiles for subcritical or supercritical open channel flow.	One-dimensional, gradually varied, steady flow conditions. Channel slopes are less than 10%.	Program is not capable of modeling movable flow boundaries such as those that occur due to sediment transport.
HEC-RAS	For computing subcritical, supercritical, and mixed flow regime open channel flow water surface profiles. Model has improved routines for evaluating the effects of obstructions such as bridges, culverts weirs, and structures in the floodplain.	Gradually varied, steady flow conditions.	
FEQ	For dynamic modeling of unsteady flow conditions in open channels and through control structures. Model can be used for a variety of stream configurations and is able to read HEC-2 and WSPRO cross-section input data.		
SWMM	For single-event or continuous simulation analysis of stormwater quantity and quality problems. Model is used most often on catchments having sophisticated storm sewer networks where accurate simulation of dynamic flow conditions is needed.	<ul style="list-style-type: none"> • Model performs best in urban areas with mostly impervious services. • Water quality simulations must be calibrated if the pollutant magnitudes are to be considered reliable. 	<ul style="list-style-type: none"> • No sub-surface water quality routing • No interaction of water quality processes except for adsorption. • Wetland water quality processes are difficult to simulate. • Scour-deposition routine is weak. • Model uses hydrologic storage routing. User needs to calculate and input the hydraulic characteristics of storage components.

2.3.2 Gravity Flow

All stormwater management facilities in the City of Naperville shall be designed for gravity-flow conditions for the appropriate design storm. Stormwater pumping facilities will only be considered if it can be demonstrated that there is no means of providing for gravity-flow drainage. The design of each stormwater management component must be based on sub-critical flow conditions. Super-critical or critical flow conditions have the potential for high velocities that are dangerous and damaging. Furthermore, these conditions are unstable and can result in widely fluctuating and unpredictable flow depths.

2.3.3 Pressure Flow

Pressure flow conditions occur in a closed-conduit system when the hydraulic grade line (HGL) is above the crown of the pipe. When it is not possible to restrict the HGL to the crown of the pipe, due to topographic constraints, the HGL may be contained within the rim, as approved by the City Engineer. Storm sewers shall be designed for the appropriate design storm based on flowing-full conditions without any pressure head. While pressure flow conditions may occur for runoff events in excess of the design storm, such conditions should be avoided by allowing excess flows to be channeled to overflow routes.

2.4 MINOR DRAINAGE SYSTEM

2.4.1. General

The minor drainage system consists of curb and gutter sections, storm sewers (with appurtenant inlets, catch basins, and manholes), swales (side yard, back yard), and small natural or man-made open channels. The function of the minor drainage system is to quickly collect and convey the runoff from the smaller, more frequent storms. Except as modified and/or appended herein, all minor drainage system components covered in this section shall be designed in accordance with the most current edition of the Illinois Department of Transportation (IDOT) Drainage Manual.

2.4.2 Swales

Side yard and rear yard swales shall be analyzed as open channels with a triangular cross-section and a mowed turf grass surface. The minimum longitudinal slope should be 2 percent. Where hardship is demonstrated, 1.5 percent may be permitted. The maximum slope allowed is 25 percent. The swale shall have sufficient hydraulic capacity to convey the 100-year design storm peak flow rate with the appropriate freeboard clearances. For residential developments, there shall be not less than 1.5 feet of freeboard between the designer high water level in the swale and the lowest adjacent top of foundation when the home is adjacent to an over land flow route. For commercial developments, there shall be no less than 1.0 feet of freeboard between the design high water level in the swale and the lowest adjacent top of foundation elevation.

Unless permission is granted by an adjacent property owner, swales must be contained entirely on the lot.

For residential properties only, window wells shall not be greater than 6 inches below the top of foundation.

2.4.3 Pavement Drainage

Maximum flow depths on any roadway shall not exceed 6 inches during the base flood condition. Inlets are to be located such that these encroachments are not exceeded, and that flow will not cross intersecting streets. These standards will apply to all local and collector streets. The following table provides equations that should be used in computing inlet spacing. The equations are based on the standard City of Naperville gutter width of 12 inches, and a recommended pavement roughness coefficient of 0.015. The maximum flow depth at the face of curb is about 1.7 inches which occurs for the steepest cross-slope of 2.5%. These equations; therefore, are applicable for both roll and barrier curb situations.

Flow Component	Pavement Cross-slope		
	3/16" per foot (1.56%)	1/4" per foot (2.08%)	5/16" per foot (2.5%)
Q Total	(2.147) S ^{1/2}	(3.068) S ^{1/2}	(3.902) S ^{1/2}
Q Gutter	(1.464) S ^{1/2}	(1.966) S ^{1/2}	(2.412) S ^{1/2}
Q Grate Capacity	(0.014) K	(0.019) K	(0.024) K

S = longitudinal pavement slope in feet per foot.

K = conveyance factor that is unique to the geometry of each grate. Values for the conveyance factors are provided in "Inlet Grate Capacities" published by the Neenah Foundry Company.

For the ponded water condition, stormwater flow bypassed from uphill inlets accumulates at a low point. At lower depths, the flow into most inlet grates takes on the characteristics of flow through a weir. As the ponding depth increases, the inlet grate opening acts more like an orifice. The equations and assumptions applicable for both types of ponded condition flow are found in the above referenced Neenah Foundry publication.

2.4.4 Storm sewers

Storm sewers shall be designed in accordance with the procedures outlined in Chapter 8 of the IDOT Drainage Manual. These procedures use the Rational Method to determine peak stormwater flow rates for both inlet spacing and storm sewer sizing.

All storm sewers that will be within the City of Naperville right-of-way shall be designed with the appropriate ASTM Class of reinforced concrete pipe (RCP). Every effort shall be made to minimize the placement of storm sewers in side yard easements. All storm sewer pipes within side yard easements shall have gasketed joints. For RCP, these joints shall be in accordance with ASTM specification C-361.

For residential properties, storm sewers shall be designed to accommodate the 10-year storm event without surcharging out of the rim. The minimum and maximum allowable subcritical design flow velocities are 2.5 fps and 10 fps, respectively. A conventional development has one or more storm sewer networks that discharge to the detention facility for the site. To avoid surcharging conditions, storm sewer outfalls to detention facilities shall be designed such that the invert of the outfall pipe is at or higher than the normal water elevation of the facility. An exception to this policy, approved by the City Engineer, will be made if the hydraulic grade line calculations can demonstrate that the design water surface is kept below the street edge of pavement elevation throughout the site.

2.4.5 Culverts

All culverts that will be within the City of Naperville right-of-way shall be designed with the appropriate ASTM Class of RCP and will have the appropriately sized precast concrete flared-end section on each end. All culverts over 12 inches in diameter shall have the appropriate grating covering the flared-end section opening.

2.5 MAJOR DRAINAGE SYSTEM

2.5.1 General

The major drainage system consists of overflow routes (streets and larger natural or man-made open channels), man-made stormwater storage facilities (basins and ponds), large natural or man-made channels, and floodplains. The major drainage system serves both conveyance and storage functions. This may occur as a result of runoff from the larger, less frequent storms and/or from an obstruction in the minor system.

2.5.2 Overflow Routes

2.5.2.1 General

Overflow (or over land flow) routes should exit a developed site in a common area that is preferably public property such as a park or public right-of-way. As a secondary choice, overflow routes can be designated along a common rear yard property line that is furthest away from homes and buildings.

2.5.2.2 Swales

Overflow route through grassed swales shall be analyzed as open channels with a triangular cross-section and a mowed turf grass surface. An overflow route swale shall have side-slopes that are a minimum of 2 percent and maximum of 25 percent. The longitudinal slope should be 2 percent. Where hardship is demonstrated, 1.5 percent may be permitted. The maximum slope allowed is 25 percent. The overflow route swale will have sufficient hydraulic capacity to convey the design storm peak flow rate with the appropriate freeboard clearances. For residential developments, there shall be no less than 1.5 feet of freeboard between the 100-Year design highwater level in the swale and the lowest adjacent top of foundation elevation. For commercial developments, there shall be no less than 1.0 feet of freeboard between the 100-Year design highwater level in the swale and the lowest adjacent top of foundation elevation.

Unless permission is granted by an adjacent property owner, swales must be contained entirely on the lot.

For residential properties only, window wells shall not be greater than 6 inches below the top of foundation.

2.5.3 Stormwater Storage

2.5.3.1 Floodplain Compensatory Storage

Existing floodplain storage will be preserved and maintained for the post-development condition. For streams with mapped FEMA Zone AE floodplain (base floodplain elevations determined and subsequently approved by FEMA), the floodplain will consist of the base flood elevation(s) set onto the approved site topography. For streams with mapped FEMA Zone A floodplain (no base floodplain elevations determined), the floodplain will need to be determined with hydrologic and hydraulic modeling acceptable to the IDNR and the City of Naperville.

2.5.3.2 Natural Depressional Storage

Isolated depressional areas can provide a significant amount of natural stormwater storage in a watershed. Therefore, they shall be included in determining the natural release rate to be compared to the allowable release rate.

2.5.4 Stormwater Detention

2.5.4.1 General

Detention requirements within the City of Naperville are subject to review under both the City's storm runoff control ordinance in addition to applicable county ordinances. Upon review, the more conservative application will apply.

The City of Naperville's storm runoff control ordinance states that detention is required for all commercial developments. Detention will also be required for residential developments of 2.5 acres or greater.

The stormwater release rate is the lesser value of the applicable County release rate or the natural release rate.

2.5.4.2 Basins

A detention basin is a facility that normally drains completely between runoff events. The maximum side-slope of a detention basin adjacent to a residential area, including multi-family developments, shall be 6 to 1 (6:1, or 6 feet horizontally for every 1-foot drop in elevation). This slope requirement applies to all sides of the basin.

The maximum side-slopes of a detention basin adjacent to a commercial area shall be 4 to 1.

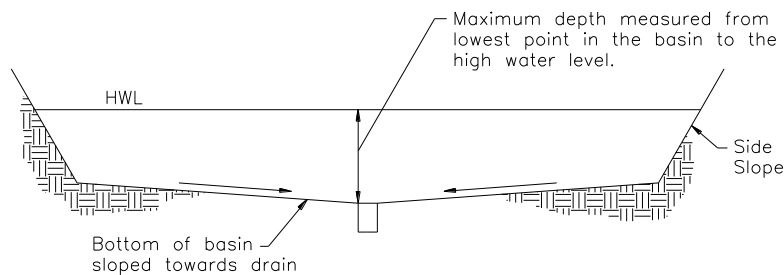
The maximum depth of stormwater storage in a detention basin shall be 6 feet. This depth is measured from the lowest point in the basin (located at the invert of the outlet pipe or at the rim elevation of an inlet grate) to the design high water elevation as illustrated below.

The bottom of a detention basin must be sloped toward the outlet or inlet grate(s) to provide drainage after it has stored stormwater. The bottom slope will be a recommended 2 percent with an allowable range of 1.5 percent minimum and 25 percent maximum.

Naturalized detention basins with minimal to no slope are allowed by the City if its primary purpose is for best management practices (BMPs).

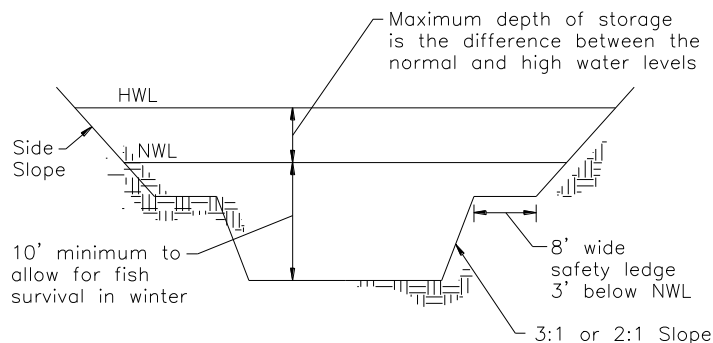
2.5.4.3 Ponds

A detention pond is a facility that maintains a fixed minimum water elevation between runoff events. This definition excludes the water volume losses due to infiltration, evaporation, or de-watering due to periodic maintenance. The maximum side-slopes of a detention pond adjacent to a residential area, including multi-family developments, shall be 6 to 1 (6:1, or 6 feet horizontally for every 1-foot drop in elevation). The maximum side-slopes of a detention pond adjacent to a commercial area shall be 4 to 1. The maximum depth of stormwater storage in a detention pond shall be 6 feet. This depth is measured from the normal water level elevation to the design high water elevation.



TYPICAL DRY DETENTION BASIN

At least one quarter of the detention pond area (as measured at the normal water level) shall have a minimum depth of 10 feet below the normal water level to provide a refuge for fish to survive during the winter.



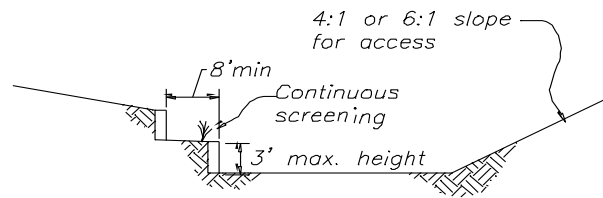
TYPICAL WET (RETENTION) BASIN

A detention pond must have an 8-foot-wide safety ledge located 3 feet below the normal water level where emergent plants are not desired, unless a justification is otherwise provided and approved by the City Engineer. This safety ledge provides a stopping point for someone who may have inadvertently fallen into the water.

2.5.4.5 Retaining Walls Used for Detention Facilities

Retaining walls are not permitted in or adjacent to single- family residential developments.

Where permitted, retaining walls around stormwater detention facilities shall not exceed three feet in height. If a wall greater than 3 feet in height is required, the wall must be terraced after the first 3-foot-high lift. A minimum horizontal space of 8 feet will be required between wall lifts. A landscape buffer will be required around the back of the bottom retaining wall as illustrated below.



PROPOSED RETAINING WALL SECTION

2.5.4.6 Parking Lot Detention

A maximum depth of 1 foot will be allowed for stormwater storage on parking lots. While allowed, other storage options are preferred.

2.5.4.7 Underground Detention

Underground storage is allowed if there is no practical alternative to construct a surface detention facility. The City will require that the owner of an underground detention facility have a qualified engineer inspect the system and furnish an inspection report to the City Engineer on request. The report will include a certification that the condition of the detention system is such that it will operate in accordance with the original design intent. The Plat of Dedication and/or Plat of Easement for the development will include specific language to cover this requirement.

2.5.4.8 Fee-in-Lieu-of Detention

A fee may be paid in lieu of a detention basin or pond if it can be demonstrated that site conditions are such that there is no practical means of constructing such a facility and that there will be no adverse impacts to areas downstream of the site if a detention facility is not provided. The Naperville City Council must approve this alternative before it can be implemented. The fee is computed as the product of the watershed fee multiplied by the acre-feet of storage that would be required for the proposed site development. This fee may be adjusted with time; therefore, the current fee amount should be confirmed with the City Engineer.

2.5.4.9 Location of Detention Facilities

The minimum separation between the right-of-way of a public street and the high-water level of a detention basin or pond is 10 feet plus one and one-half times the maximum design depth of the detention facility.

Adjacent to a State route, IDOT requirements apply.

2.5.4.10 Use of Public Streets for Detention Prohibited

Public street pavements shall not be used for stormwater detention storage.

2.5.4.11 Stormwater / BMP Easements

Stormwater management facilities and PCBMPs shall have dedicated easements assigned to them. The easements shall be shown and called out on the Final Engineering Plans and Plat of Subdivision and recorded in the county where they are located.

2.6 FLOODPLAIN MANAGEMENT

2.6.1 Development Standards

For streams with mapped FEMA Zone AE floodplain (base floodplain elevations determined and subsequently approved by FEMA), the floodplain will consist of the base flood elevation(s) set onto the approved site topography. For streams with mapped FEMA Zone A floodplain (no base floodplain elevations determined), the floodplain will need to be determined with hydrologic and hydraulic modeling acceptable to the IDNR and the City of Naperville with the resulting floodplain delineation based on the approved site topography.

2.6.2 LOMR Requirements

For streams with mapped FEMA Zone A floodplain, there will be, in most cases, a discrepancy between the FEMA delineated Zone A floodplain and the newer, refined delineation. Both delineations need to be shown on the final engineering master grading plan. New development will be allowed in FEMA delineated Zone A floodplain areas that are not part of the approved refined delineation. The developer will be required to apply for and receive a FEMA Letter of Map Revision (LOMR) before the City will issue building permits for the habitable structures proposed for these areas. The City of Naperville will require that the builder submit a FEMA Elevation Certificate for these structures at the time that the Final Grading Survey (FGS) is submitted.

SECTION 3 SANITARY SEWER DESIGN

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SECTION 3 SANITARY SEWER DESIGN

3.1 GENERAL DESIGN CRITERIA

3.1.1 Requirement for Sanitary Sewer

All sanitary sewage of domestic and other water borne wastes shall be collected and conveyed in a sanitary sewer pipe system to a point of discharge into an existing sanitary sewer system, City of Naperville interceptor, or sewage treatment plant. No sanitary sewage shall be allowed to enter any storm sewer system or discharge onto the ground or into receiving streams without first being treated in accordance with city, county, state and federal regulations.

3.1.2 Design Approval

The Department of Public Utilities-W/WW shall approve all designs and shall alter the following design requirements as necessary to meet the City Utilities Master Plan, and as they comply with Section 300 of the Naperville Specifications for Construction and Standard Details. The Department of Public Utilities-W/WW and the Transportation Engineering and Development Business Group shall review and comment on all designs.

All newly installed, public sanitary mains are required to be televised at the developer/builders cost and approved by DPU-W/WW before acceptance.

Proposal of new sanitary sewer lift stations requires approval from the Director of Public Utilities, prior to the design and review process.

The City of Naperville's standard details, as currently adopted by the Department of Public Utilities, should be used in all construction plans. Any modifications to these standard details require approval from the Director of Public Utilities-W/WW.

All new single family residential and/or Commercial Development sanitary sewer laterals shall be installed to conform with section 302 of the Naperville Standard Specifications for Construction.

One sanitary service is permitted per building unless permission is obtained from the department.

3.1.3 IEPA Permit Required

All public sanitary sewer mains require an IEPA Construction permit number and an Illinois Pollution Control Board permit number, prior to construction. Some private extensions may require a permit, dependent upon the design population equivalent.

3.1.4 Differentiation Between Public and Private Sanitary Sewers

Those portions of the sanitary sewer system that are located within the public right-of-way are the responsibility of the City. The City shall also maintain public mains, serving more than one customer. The maintenance and repair costs for the sanitary sewer system located on private property, beyond the limits of the right-of-way, are the responsibility of the property owner or property owner's association depending on the associate's covenants and guidelines. In such cases where the City performs

maintenance or repairs on the private sanitary sewer system, the City reserves the right to charge the property owner for necessary work.

All engineering plans shall clearly differentiate between public and private sanitary sewers.

3.1.5 Easements

All public sanitary sewers that are not located within a publicly dedicated right-of-way shall be placed in a public utility and drainage easement, minimum 15 feet wide or as directed by the Department of Public Utilities, to the City of Naperville. The easement shall be granted to the City either through a recorded plat of subdivision or a recorded plat of easement. The City shall be granted access to these easements if not directly adjacent to public right-of-way. An easement along the service line to the inspection manhole shall also be provided.

At the discretion of the Director of Public Utilities, the City may require additional easements for future maintenance or repair of sanitary sewers, even those sewers that may be located within the public right-of-way. For example, the City may have an extra deep sanitary sewer located within the public right-of-way. However, the City may require a public utility and drainage easement parallel to the edge of the right-of-way to accommodate future repair of the sanitary sewer if it ever needs to be excavated and repaired.

3.2 SANITARY SEWERS

3.2.1 Sanitary Sewer Pipe Materials

All sanitary sewer pipe materials and appurtenances shall be in conformance with Section 302 of the City of Naperville Standard Specifications.

3.2.1.1 Sanitary Service Laterals.

A minimum size of 6" is required for all sanitary service laterals. Pipe shall conform to section 302.3 and the standard details of the Naperville Standard Specifications for Construction. Connection to a CIPP lined sanitary main shall be installed per standard detail 390.21 Re-use of an existing Vitrified Clay Pipe (VCP) sanitary lateral shall not be approved unless it is CIPP lined from the main to the cleanout including the main line connection per DPU-W/WW requirements, televised after installation, and approved by DPU-W/WW staff. The main line CIPP connection must be full circumference. The CIPP lateral lining must meet the requirements of ASTM F2561-20 and F3240-17, as manufactured by LMK Technologies or approved equal.

3.2.2 Location in the Public Right-Of-Way

Sanitary sewers shall be located within the public right-of-way as directed by the City Engineer. In general, sanitary sewers shall be located 7.5 feet inside the right-of-way on the south and east sides of the right-of-way.

3.2.3 Curvilinear Alignment of Sanitary Sewers

Curvature of sanitary sewers is allowed for sewers 8 inches to 12 inches in diameter. Alignments must follow the general alignment of streets. Only a simple curve design is acceptable. The minimum allowable radius of curvature is 300 feet. Compression type

pipe joints are required and manholes are required at the beginning and end of all curves. Maximum joint deflection shall not exceed the manufacturer's recommendations.

3.2.4 Sewer and Water Main Separation

Sanitary sewers and services that are laid in the vicinity of pipelines designated to carry potable water shall meet the conditions set forth in Section 5 of this manual.

3.2.5 Depth of Pipe Cover

All pipe shall be laid to a minimum depth of 7 feet measured from the proposed ground surface to the top of the pipe, unless specifically allowed otherwise in special circumstances by the Director of Public Utilities. If allowed, sanitary sewer and services with ground cover less than 4 feet or more than 25 feet must be constructed of ductile iron class 50 pipe with polywrap. All sanitary sewers and services with less than 4 feet of cover shall be insulated with a 2-inch exterior grade rigid insulation board. The insulation shall have a minimum R-value of R-9, and comply with ASTM C 578-92 Type 1X.

3.2.5.1 Overhead Sewers

Overhead sewers shall be required for all new construction, as well as major additions and basement remodels.

3.2.6 Sanitary Sewer Sizing

Sewer size shall be designed on the basis of a design average flow of not less than 100 gallons per capita per day and provide a minimum of 2.0 feet per second velocity when flowing full. The Director of Public Utilities may increase sewer size in accordance with Section 3.2.7 noted below. In no case shall a public sewer be sized less than 8 inches in diameter.

3.2.7 Oversizing and Extra Depth Requirements

The Director of Public Utilities-W/WW may request that sanitary sewers either be oversized or installed at an additional depth in order to provide service to additional benefiting properties. Section 7-3-6 of the Municipal Code (City participation in construction of public improvements) outlines the City's policy relative to cost sharing for oversizing and extra depth installation.

If there is a significant change in land use, sanitary sewer sizing calculations may be required. The existing sanitary sewer may need to be upsized to provide adequate conveyance for changes in land use.

3.2.8 Sanitary Sewer Slopes

All sanitary sewer slopes shall meet the requirements of the following sections:

3.2.8.1 Minimum Slopes

Sanitary sewers shall be designed such that the minimum slopes are not less than the following:

Pipe Diameter	Minimum Slope	Desired Slope
6 inch	1.00%	1.00%
8 inch	0.40%	0.45%
10 inch	0.28%	0.30%
12 inch	0.23%	0.25%

3.2.8.2 Maximum Slopes

Sanitary sewers shall be designed such that the slopes do not exceed a maximum of 12%, and a maximum velocity of 10 fps. If the sanitary sewer system cannot be designed without exceeding a slope of 12%, then drop manhole assemblies shall be utilized.

3.2.8.3 Last Run of Manhole Sets

On last runs of all manhole sets, a minimum 1% slope needs to be provided in order to provide adequate flushing due to low flows.

3.2.9 Limits of Installation

At a minimum, sewers shall extend across the frontage of the property, at the developer's cost, such that a connection can be made with minimal disturbance in the future. In some cases, the City may require that the sanitary sewer be installed from one corner to the diagonally opposite corner, at the developer's cost.

3.3 SANITARY SEWER MANHOLES

Manholes for sanitary sewers shall have a minimum inside diameter of 48 inches and shall be constructed of pre-cast concrete units in accordance with ASTM C 478 and Section 32 of the "Standard Specifications for Water and Sewer Main Construction in Illinois," latest edition, and shall follow the City of Naperville sanitary sewer standards.

3.3.1 Manhole Location and Spacing

Manholes shall be located at the junction of two sanitary sewer pipes or at any change in grade, alignment, material, or size of pipe. The maximum spacing of manholes shall be 500 feet, or as approved by the Director of Public Utilities.

In general, the City of Naperville prefers to minimize the number of manholes needed for a project. This will help reduce future operation and maintenance costs.

3.3.2 Invert Elevations in Manholes

Inverts of similar size pipe are to match other inverts. Where a smaller pipe intersects a larger pipe, the spring line or top of pipe of both pipes shall be at the same elevation, unless otherwise directed by the Director of Public Utilities. This is also to be done when tapping (core and boot required) into an existing manhole.

A 0.1% slope must be maintained across the manhole.

Service laterals shall not connect directly to the manhole without approval by Utility Manager. Should approval be granted, the bench shall be re-constructed to form a channel for proper discharge.

3.2.3 Drop Manholes

Drop manhole assemblies shall be provided at the junction of sanitary sewers where the difference in grade is in excess of 2 feet. The drop assembly shall follow Naperville Standards with filleted inverts. Drops are to be made outside of the structure unless otherwise approved by the Naperville Department of Public Utilities-W/WW. A minimum of 24 inches between the inverts of the drop assembly must be provided. If the difference between the inverts is less than 24 inches, the inverts must match.

Use of drop manholes must be approved by DPU-W/WW utility manager.

When pipe inverts in a manhole do not meet the requirements of section 3.3.2, a drop manhole assembly shall be used.

3.3.4 Requirement for Inspection Manholes and Clean-Outs

All commercial, office, institutional, industrial, manufacturing, and other non-residential buildings shall have an inspection manhole located outside of the building that will allow the City to observe the discharge from the building into the public sanitary sewer system.

An inspection manhole is required for any multi-family building that has more than six (6) units. Additionally, clean-outs shall be required on multi-family services serving between two (2) and six (6) units.

A clean-out will also be required for any service line over 90 feet in length which does not have an inspection manhole.

3.3.5 Grease/Oil/Sand Trap Manholes

Grease/oil/sand trap manholes, as required by the Illinois Plumbing Code, shall be shown on the engineering plans. Sizing calculations demonstrating compliance and a detail sheet in accordance with the Illinois Plumbing Code will be required.

3.4 APPURTENANCES

3.4.1 Casing Pipes

Manufactured non-metallic or non-corrosive casing spacers, adjustable runners, or cradles shall be used to support the pipe in the casing. A minimum of two supports shall be used per pipe for lengths up to 12.5 feet, and a minimum of three supports shall be used for lengths greater than 12.5 feet, or per manufacturer's recommendation. The annular space shall be filled with pea gravel or as required by permitting agency, and provisions shall be made so that no voids are left to prevent flotation.

3.4.1.1 Casing Pipe Material

The steel casing pipe shall be bituminous coated, a minimum of 30 mils thickness inside and out, and shall be of leak proof construction, capable of withstanding the anticipated loadings. The steel casing pipe shall have a minimum yield strength of 35,000 psi and shall meet the requirements of ASTM A139, Grade B. Ring deflection shall not exceed 2% of the nominal diameter. The steel casing pipe shall be delivered to the jobsite with beveled ends to facilitate field welding.

<u>Steel Casing Diameter</u>	<u>Minimum Wall Thickness (Inches)</u>
20" and 22"	0.344
24"	0.375
28"	0.438
30"	0.469
32"	0.501
34" and 36"	0.532

3.4.1.2 Sizing of Casing Pipes

The diameter of the casing pipe shall be a minimum of 12 inches greater than the outside nominal diameter of the sewer.

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SECTION 4 STREET DESIGN

- 4.1 GENERAL DESIGN CRITERIA**
- 4.2 ROADWAY GEOMETRICS**
 - 4.2.1 Functional Classification**
 - 4.2.1.1 Major Arterial**
 - 4.2.1.2 Minor Arterial**
 - 4.2.1.3 Collector**
 - 4.2.1.4 Neighborhood Connector**
 - 4.2.1.5 Local Streets**
 - 4.2.2 Minor Street Design Guidelines**
 - 4.2.2.1 Pavement Width**
 - 4.2.2.2 Maximum Cul-de-Sac Length**
 - 4.2.2.3 Curbs and Gutters**
 - 4.2.2.4 Sidewalks**
 - 4.2.2.5 Street Lighting**
 - 4.2.2.6 Street Name Signs**
 - 4.2.2.7 Centerline Radii of Horizontal Curves**
 - 4.2.2.8 Pavement Design**
 - 4.2.2.9 Pavement Structural Numbers**
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- 4.3 INTERSECTION DESIGN**
- 4.4 SIDEWALKS**
 - 4.4.1 Minimum Width**
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- 4.5 STREET LIGHTING**
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 - 4.5.3.2 Controller and Power Source**
 - 4.5.4 Lighting Circuit Design**
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SECTION 4 STREET DESIGN

4.1 GENERAL DESIGN CRITERIA

This section of the Design Manual generally follows the guidelines as established by the Institute of Transportation Engineers (ITE), the Illinois Department of Transportation (IDOT) and the American Association of State Highway and Transportation Officials (AASHTO). The references used to establish this Design Manual include the following:

- *Guidelines for Major Street Design* (ITE Publication No. RP-010A)
- *Guidelines for Residential Subdivision Street Design* (ITE Publication No. RP-011C)
- *A Policy of Geometric Design of Highways and Streets* (AASHTO, latest edition)
- *Illinois Department of Transportation Bureau of Local Roads and Streets Manual*
- *Illinois Department of Transportation Bureau of Design & Environment Manual*
- Illinois Accessibility Code

This chapter presents modifications to these guidelines in accordance with the policies and procedures established by the City of Naperville.

4.2 ROADWAY GEOMETRICS

4.2.1 Functional Classifications

4.2.1.1 Major Arterial

These streets are used primarily to carry the heavier traffic pattern providing continuity throughout the contiguous urban area. Access control will be maintained through limiting access to intersections with other roads.

4.2.1.2 Minor Arterial

A street used primarily for intersections of sections of the City and deemed desirable for construction of other public utilities within the street right of way.

4.2.1.3 Collector

A street used primarily to provide ready collection of traffic from residential areas and to convey this traffic to the major arterial and highway system. Even though this street may carry some through traffic, its primary function is to feed traffic to the arterial streets and to provide local access.

4.2.1.4 Neighborhood Connector

Connect residential and local streets within a neighborhood to Collector streets and to the Arterial street network. All neighborhoods have at least one neighborhood connector street, and many have two or more.

4.2.1.5 Local Streets

The primary function is to serve adjoining property. They shall be arranged to conform to the topography, to discourage use by through traffic, and to provide access to abutting property.

4.2.2 Minor Street Design Guidelines

A minor street is defined as any street classified below the functional classification of an arterial. The design guidelines to be used in developing the minor street system layout shall be in accordance with Appendix E of "*Guidelines for Residential Subdivision Street Design*" prepared by the Institute of Transportation Engineers, except as otherwise noted in the following provisions:

4.2.2.1 Pavement Width

The minimum pavement width for all local streets and cul-de-sacs is 28 feet, measured from back of curb to back of curb. The minimum pavement width for collector streets is 37 feet, measured from back of curb to back of curb. Neighborhood connector streets shall either be 28 feet or 37 feet depending upon the discretion of the City Engineer. Neighborhood connectors shall be 37 feet adjacent to all school sites, park sites or other major facilities and on approaches to all collector and arterial roadways.

4.2.2.2 Maximum Cul-de-Sac Length

The length of a cul-de-sac should not exceed 1,000 feet, as measured from the centerline of the intersecting street, along the centerline to the center of the bulb of the cul-de-sac. For lengths in excess of 1,000 feet, a secondary point of access must be provided.

4.2.2.3 Curbs and Gutters

The standard curb and gutter section for all cul-de-sac and local roads is a type M 3.12 mountable rolled curb. The curb and gutter section for all collector streets shall be a barrier type B 6.12 curb and gutter. Neighborhood connector streets can be constructed with either M 3.12 or B 6.12 curb and gutter. In general, the M 3.12 curb and gutter will be used in the 28-foot wide sections of neighborhood connector streets while the B 6.12 curb and gutter will be used on the 37-foot wide sections.

4.2.2.4 Sidewalks

A 5-foot sidewalk is required on both sides of all streets and cul-de-sacs. Pedestrian circulation is to be provided to separate vehicular traffic from pedestrians. Curb ramps which connect to the road network shall have receivers on the opposite side of the street.

4.2.2.5 Street Lighting

Street lighting, including underground service cable, is required throughout all subdivisions for all public streets.

4.2.2.6 Street Name Signs

Street name signs are required at all street intersections. The developer will be responsible for procurement and installation of initial street signs following approval by City review.

4.2.2.7 Centerline Radius of Horizontal Curves

Horizontal curves should be as large as practical, particularly for collector and arterial streets. For local and neighborhood connector streets, the centerline radius of midblock horizontal curves will be less than or equal to 100 feet or greater than

300 feet with a minimum radius of 65 feet. For collector streets, the minimum centerline radius of horizontal curves is 450 feet.

4.2.2.7 Pavement Design

All streets within the subdivision shall be surfaced with either bituminous concrete or Portland cement concrete. The pavement structure shall be designed according to the Sections 502, 503, and 504 of the City of Naperville Standard Specifications.

4.2.2.8 Pavement Structural Numbers

The City of Naperville has established minimum pavement structural numbers for various classifications of roadways. These values are minimums based on expected vehicle volumes and loading. Variance from these values will require approval from the City Engineer.

Pavement Classification	Minimum Structural Number
Parking Lot (multi-family residential or standard commercial)	2.50
Local Street	2.98
Collector	4.00
Arterial	By Design

The typical section for a local street is based on 1 ½ inch HMA surface course, 2 ½ inch HMA binder course, and 12-inch aggregate base course.

Neighborhood Connector streets will be determined by City Staff to follow Local Street or Collector standard based on expected use and volumes.

In order to accomplish these designs, the following structural coefficients shall be used to calculate a roadway’s structural number. All classifications excluding parking lots shall provide a minimum 4 inches of HMA pavement course. The values are presented per inch of pavement section thickness.

Pavement Section	Structural Coefficient
Hot Mix Asphalt Surface Course	0.40
Hot Mix Asphalt Binder Course	0.33
Aggregate Base Course	0.13
Other construction	By Staff approval

Concrete pavements are allowed but require approval from the City Engineer.

4.2.3 Right-of-Way Requirements

Type of Roadway	Minimum ROW (FT)
Major Arterial	120
Minor Arterial	100
Collector	80
Neighborhood Connector	Varies from 66 to 80
Local Street	66
Cul-de-Sac Bulb	62
Alley	30

4.3 INTERSECTION AND RESIDENTIAL STREET DESIGN

The design guidelines follow the “*Guidelines for Residential Subdivision Street Design*” as published by the Institute of Transportation Engineers. Table 3: Intersection Design Guidelines denotes three classifications for the type of terrain. In the City of Naperville, the terrain is generally level (has a longitudinal slope of 0-8%). Therefore, the guidelines under this classification should be followed.

Criteria	Requirement
Approach Speed (mph)	25
Clear Sight Distance (length along each approach leg) (feet)	90
Minimum Angle of Intersection	70° (90° Preferred)
Minimum Curb Radius (feet)	
a. Local-Local	20
b. Local-Neighborhood Connector	25
c. Local-Collector	25
d. Neighborhood Connector-Neighborhood Connector	30
e. Neighborhood Connector-Collector	30
f. Neighborhood Connector-Arterial	30
g. Collector-Collector	30
h. Collector-Arterial	30
Minimum Centerline Offset of Adjacent Intersection (feet)	
a. Local-Local	125
b. Local-Neighborhood Connector	150
c. Local-Collector	150
d. Neighborhood Connector-Neighborhood Connector	200
e. Neighborhood Connector-Collector	200
f. Collector-Collector	200
Minimum Tangent Length (feet)	
Approaching Intersection (each length)	50

4.4 SIDEWALKS

4.4.1 Minimum Width

The minimum width for sidewalks is 5 feet, except where the volume of pedestrian or bicycle traffic justifies a greater width, or where parked vehicles may overhang onto the sidewalk. At locations where carriage walk is required, the sidewalk width should be expanded to 7 feet where possible. The minimum sidewalk width in the Central Business District is 6 feet (refer to the City of Naperville Downtown Streetscape standards, latest edition, for additional guidance).

4.4.2 Location

All sidewalks are to be located such that the outside edge of the walk is 1-foot from the dedicated right-of-way line, and entirely within the dedicated right-of-way. Where carriage walks are being reconstructed, they should be placed a minimum 3 feet from back of curb.

4.4.3 Lateral Separation

The minimum lateral separation between the edge of a sidewalk and any above ground obstruction (i.e., posts, poles, tree trunks, utility boxes, etc.) shall be 1-foot.

4.4.4 Slope and Grade

Transverse Slope, minimum 0.5%

Transverse Slope, maximum 2.0%

Longitudinal Slope, maximum 5% (20:1)

Longitudinal Slope, in excess of 5% shall meet the requirements of the Illinois Accessibility Code.

4.4.5 Intersections

When a sidewalk approaches an intersection with a street to provide a pedestrian crossing; the curb and adjacent sidewalk shall be depressed, meeting the requirements of the Illinois Accessibility Code. Refer to the City of Naperville Standard Detail PVMT 31.

Sidewalks are to be continuous through paved areas such as driveways and parking lot entrances.

4.5 STREET LIGHTING

4.5.1 Performance Standards

Street lighting within the City of Naperville roadways (local, neighborhood connector, collector, arterial, and Central Business District (CBD)) shall meet the performance standards as described below. Lighting on County and State owned and/maintained roadways shall meet the lighting levels as required by the County and State.

4.5.2 Approval

Street lighting design within the City of Naperville is subject to review. Catalog cuts for all components of the street lighting system must be submitted for review and approved prior to installation. The components must meet the requirements set forth in Section 600 of the City of Naperville Standard Specifications. Submittals include, but are not

limited to: poles, luminaire arms, luminaires, breakaway bases, photo-cells, conduit, cable, fuses, handholes, ground rods, foundations, controllers and labels.

4.5.3 Lighting Power Source

4.5.3.1 Direct Connected Power Source

Street lighting controllers are not required for local streets and cul-de-sacs. Street lights shall be fed from the nearest power source as indicated by the City of Naperville Department of Public Utilities/Electric. Up to eight (8) street lights can be connected in series out in one direction from a single power source. Each street light shall also be equipped with a photocell.

4.5.3.2 Controller and Power Source

All street lights on neighborhood connector, collector, arterial and CBD streets shall be run from a street lighting controller meeting the specifications noted in the City Standards. In general, the controller shall be located at the mid-point of all of the street lights run by the controller. The controller shall be 120/240V single phase with a 100A rating in non-CBD areas and 225A in CBD area. The power source shall be determined by the City of Naperville Department of Public Utilities-Electric. Street lights should alternate circuits. A photocell shall be placed on the street light nearest the controller and connect back to the controller.

4.5.4 Lighting Circuit Design

Branch circuit shall have alternating circuits. Voltage drop calculations shall be provided for all circuits. Voltage drop shall not exceed 10 percent at the furthest light pole.

4.5.5 Design Standard Summary

Light pole types, luminaire manufacture/model numbers, pole spacing/layout and circuit configurations shall be as shown in the following tables:

**CITY OF NAPERVILLE
STREET LIGHTING DESIGN STANDARDS SUMMARY SHEET 1 OF 3**

Roadway Type (Verify with the City)	Local	Neighborhood Connector	
Min ROW	66'	66'	80'
Typ. Roadway Width (BOC to BOC)	28'	28'	37'
Standard Detail #	690.01	690.01	690.01
Pole Mounting Height	23'	23'	23'
Arm Type	Davit	Davit	Davit
Arm Length	8'	8'	8'
Pole Layout	Single Side or Staggered	Single Side or Staggered	Single Side or Staggered
Maximum Pole Spacing	130' (Note 5)	150' (Note 5)	150' (Note 5)
Luminaire Type	Cobra Head	Cobra Head	Cobra Head
CCT	3000K	3000K	3000K
Wattage	23W	40W	40W
Lumens	3,102	5,360	5,360
Type	Type II	Type II	Type II
Pole & Luminaire Color	Bronze	Bronze	Bronze
Warranty	10 years	10 years	10 years
Luminaire Manufacturer and Model # (Note 6)	Acuity #ATBX-P30-MVOLT-R2-3K-BZ-NL-P7	Acuity #ATBS-P10-MVOLT-R2-3K-BZ-NL-P7	Acuity #ATBS-P10-MVOLT-R2-3K-BZ-NL-P7

**CITY OF NAPERVILLE
STREET LIGHTING DESIGN STANDARDS SUMMARY SHEET (2 of 3)**

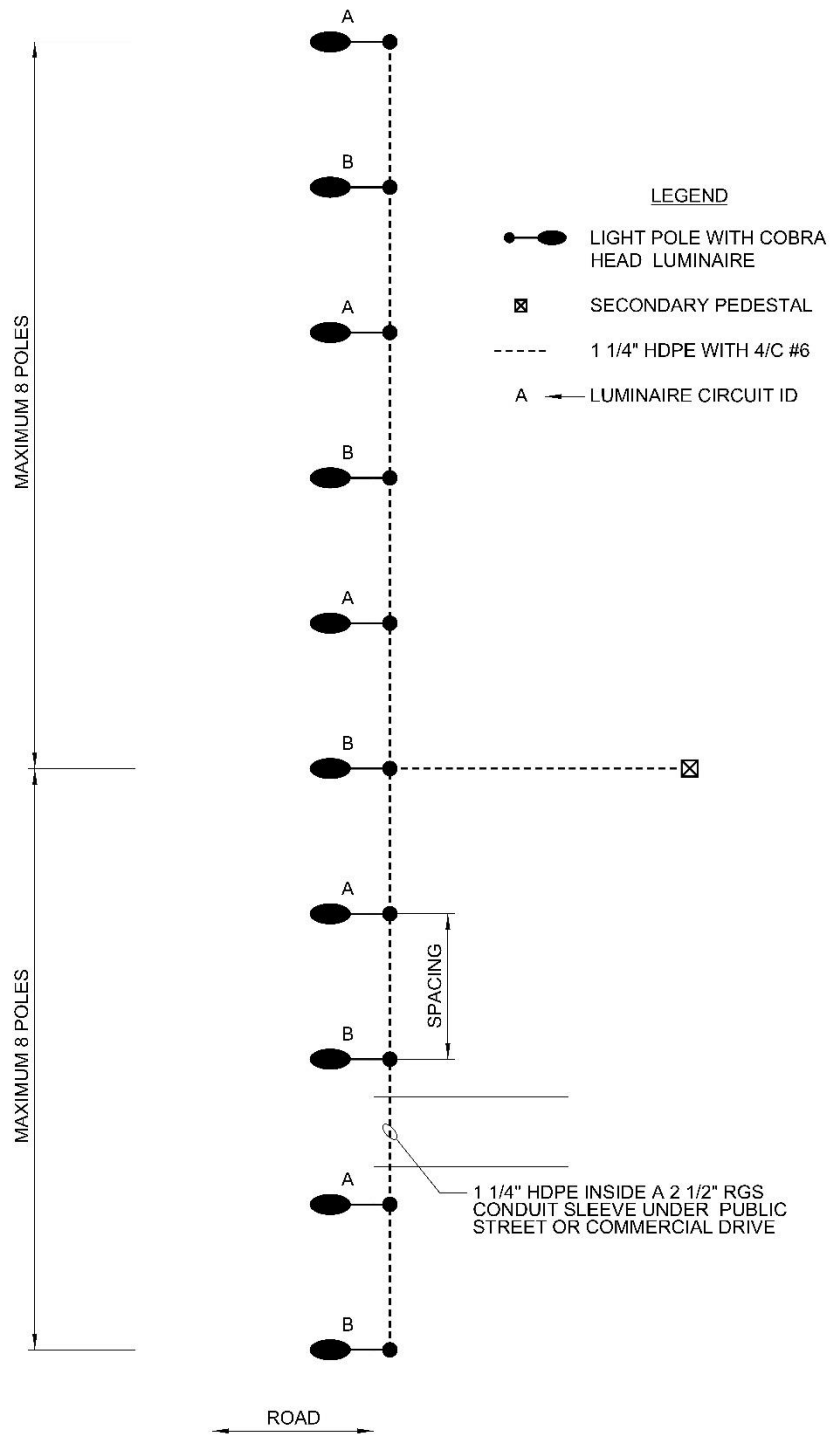
Roadway Type (Verify with the City)	Collector (Residential)			Collector (Non-Residential)		
Min ROW	80'	80'	80'	80'	80'	80'
Typ. Roadway Width (BOC to BOC)	37'	60'	37'	37'	60'	60'
Standard Detail #	690.02	690.02	690.03	690.04	690.04	690.05
Pole Mounting Height	32'	32'	32'	32'	32'	32'
Arm Type	Davit	Davit	Twin Davits	Truss	Truss	Twin Trusses
Arm Length	10'	10'	10'	10'	10'	10'
Pole Layout	Single Side or Staggered	Opposite	Median	Single Side or Staggered	Opposite	Median
Maximum Pole Spacing	150'	180'	150'	150'	180'	180'
Luminaire Type	Cobra Head	Cobra Head	Cobra Head	Cobra Head	Cobra Head	Cobra Head
CCT	3000K	3000K	3000K	3000K	3000K	3000K
Wattage	92W	92W	92W	92W	92W	92W
Lumens	11,050	11,050	11,050	11,050	11,050	11,050
Type	Type II	Type II	Type II	Type II	Type II	Type II
Pole & Luminaire Color	Bronze	Bronze	Bronze	Natural Aluminum	Natural Aluminum	Natural Aluminum
Warranty	10 years	10 years	10 years	10 years	10 years	10 years
Luminaire Manufacturer and Model # (Note 6)	Acuity #ATBS-P60-MVOLT-R2-3K-BZ-NL-P7	Acuity #ATBS-P60-MVOLT-R2-3K-BZ-NL-P7	Acuity #ATBS-P60-MVOLT-R2-3K-BZ-NL-P7	Acuity #ATBS-P60-MVOLT-R2-3K-NL-P7	Acuity #ATBS-P60-MVOLT-R2-3K-NL-P7	Acuity #ATBS-P60-MVOLT-R2-3K-NL-P7

**CITY OF NAPERVILLE
STREET LIGHTING DESIGN STANDARDS SUMMARY SHEET (3 of 3)**

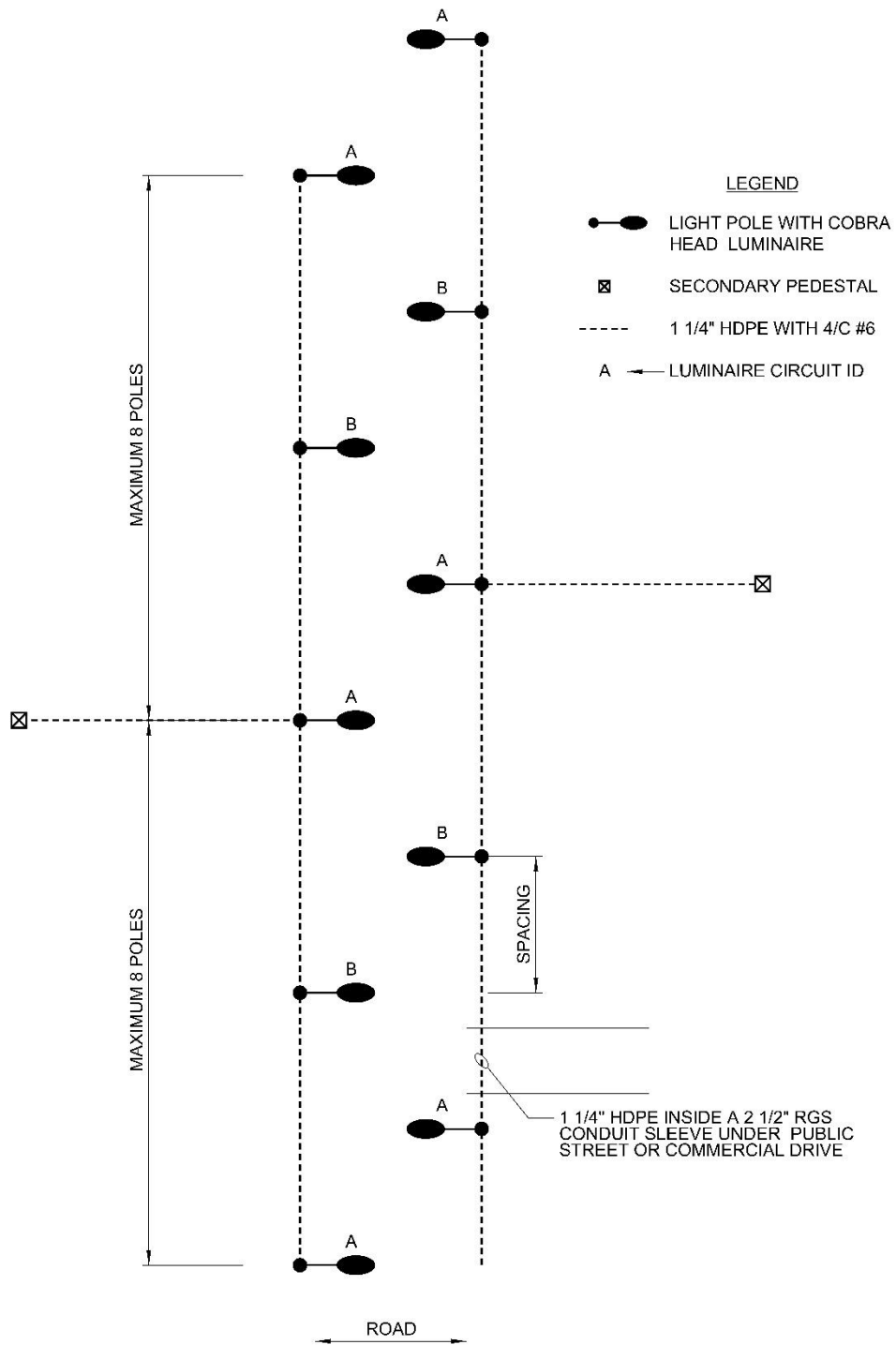
Roadway Type (Verify with the City)	Arterial			Central Business District (CBD)	
	Minor	Major			
Min ROW	100'	120'	120'	N/A	N/A
Typ. Roadway Width (BOC to BOC)	Varies	Varies	Varies	33'-53'	33'-53'
Standard Detail #	690.06	690.07	690.08	690.09	690.10
Pole Mounting Height	40'	47.5'	47.5'	30'	10'
Arm Type	Truss	Truss	Twin Trusses	Side Mounted	Post Top
Arm Length	12'	12'	12'	3"	N/A
Pole Layout	Opposite	Opposite	Median	Staggered	Opposite
Maximum Pole Spacing	Notes 1 and 4	Notes 1 and 4	Notes 1 and 4	Note 2	Note 2
Luminaire Type	Cobra Head	Cobra Head	Cobra Head	Area	Caged Acorn
CCT	4000K	4000K	4000K	4000K	4000K
Wattage	209W	209W	209W	116W	43W
Lumens	26,700	26,700	26,700	15,100	4,200
Type	Type II	Type II	Type II	Type III	Type III
Pole & Luminaire Color	Natural Aluminum	Natural Aluminum	Natural Aluminum	Black	Black
Warranty	10 years	10 years	10 years	10 years	5 years
Luminaire Manufacturer and Model # (Note 6)	Leotek #GC2-96G-MV-NW-2R-GY-700-PCR7-WL (Note 3)	Leotek #GC2-96G-MV-NW-2R-GY-700-PCR7-WL (Note 3)	Leotek #GC2-96G-MV-NW-2R-GY-700-PCR7-WL (Note 3)	General Electric #EALS-03-0-F3-AW-7-40-N-D-D1-BLCK	General Electric #EPAS-02-0-04-D-40-D-A-H-A-BLCK-037

- Notes:
1. Spacing needs to be determined by performing photometric calculations.
 2. See "Typical CBD Pole and Circuit Layout with Controller" drawing.
 3. Actual performance package needs to be determined by performing photometric calculations.
 4. Coordinate intersection lighting photometrics and design with City Traffic Engineer.
 5. Poles shall also be located at the following locations:
 - a) At each intersection, in the "stop sign" position, oriented at an angle of 90 degrees to the alignment of the street.
 - b) Inside of all horizontal curves.
 - c) On each cul-de-sac, at the point where the tangent meets the circular outside of the cul-de-sac (throat).
 - d) At the property line.
 - e) At additional locations where conditions warrant additional lighting per the City Engineer.
 6. If the equipment identified in this table is no longer available, current versions with equal or better characteristics may be substituted if approved by the City Engineer.

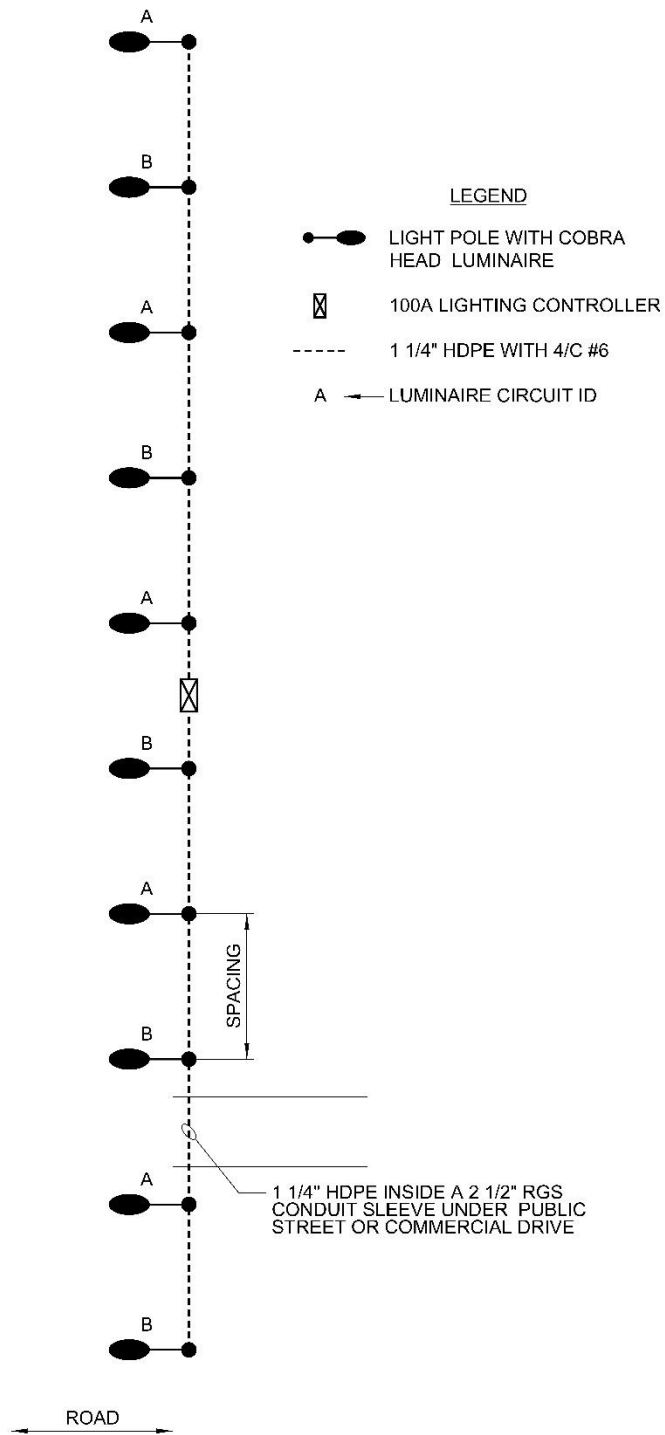
**CITY OF NAPERVILLE
TYPICAL SINGLE SIDED POLE AND CIRCUIT LAYOUT WITH DIRECT CONNECT**



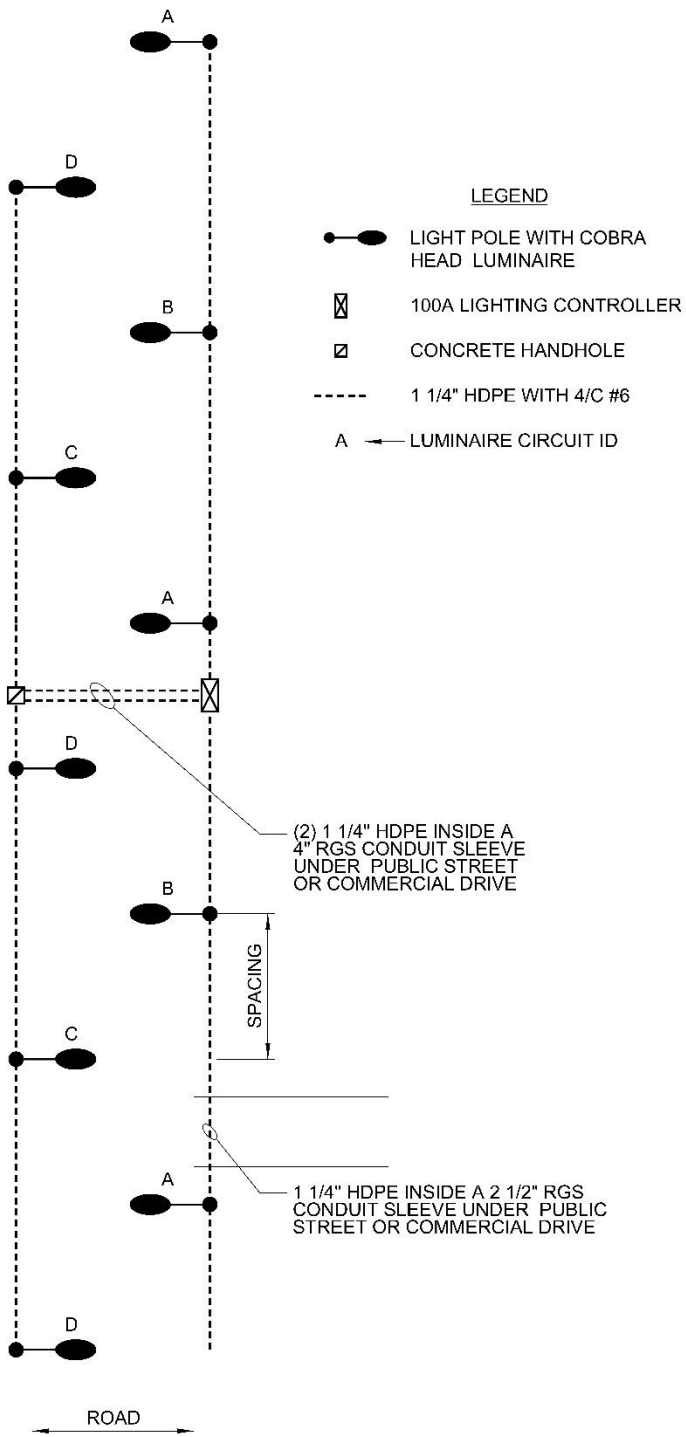
**CITY OF NAPERVILLE
TYPICAL STAGGERED POLE AND CIRCUIT LAYOUT WITH DIRECT CONNECT**



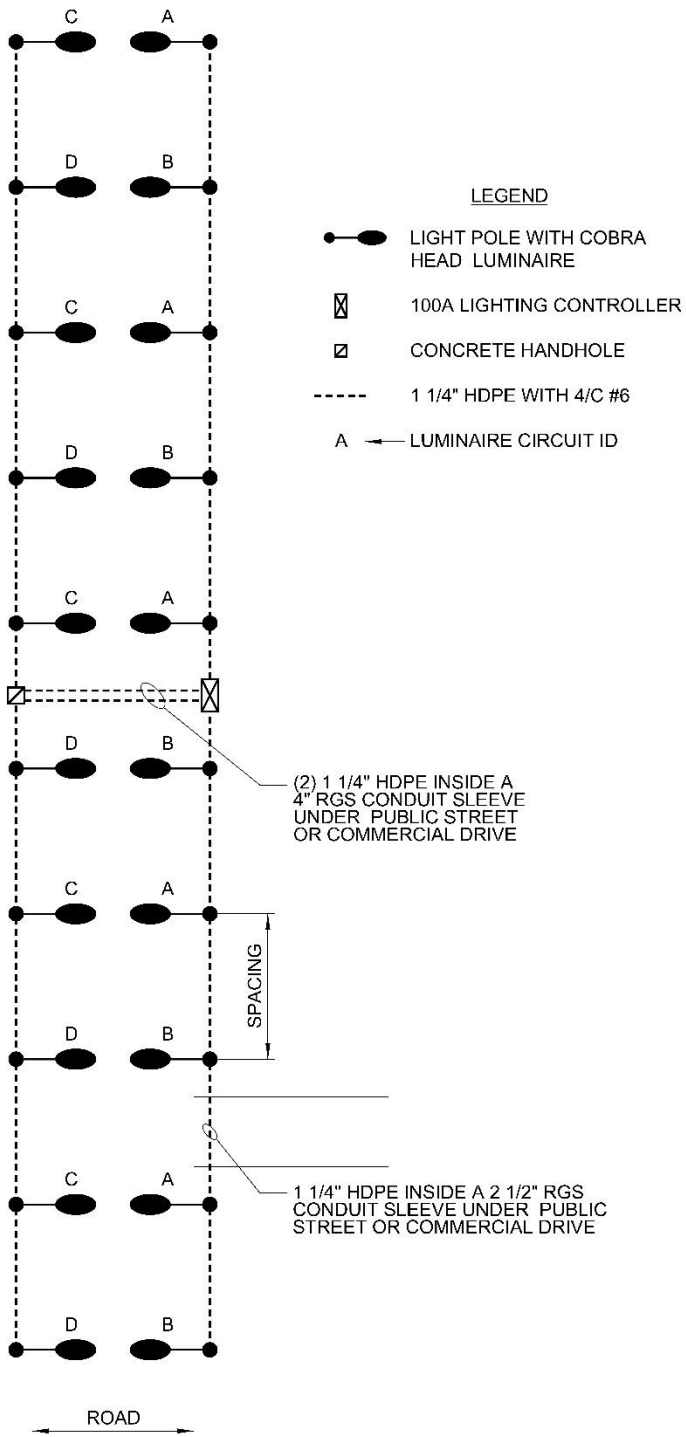
**CITY OF NAPERVILLE
TYPICAL SINGLE SIDED POLE AND CIRCUIT LAYOUT WITH CONTROLLER**



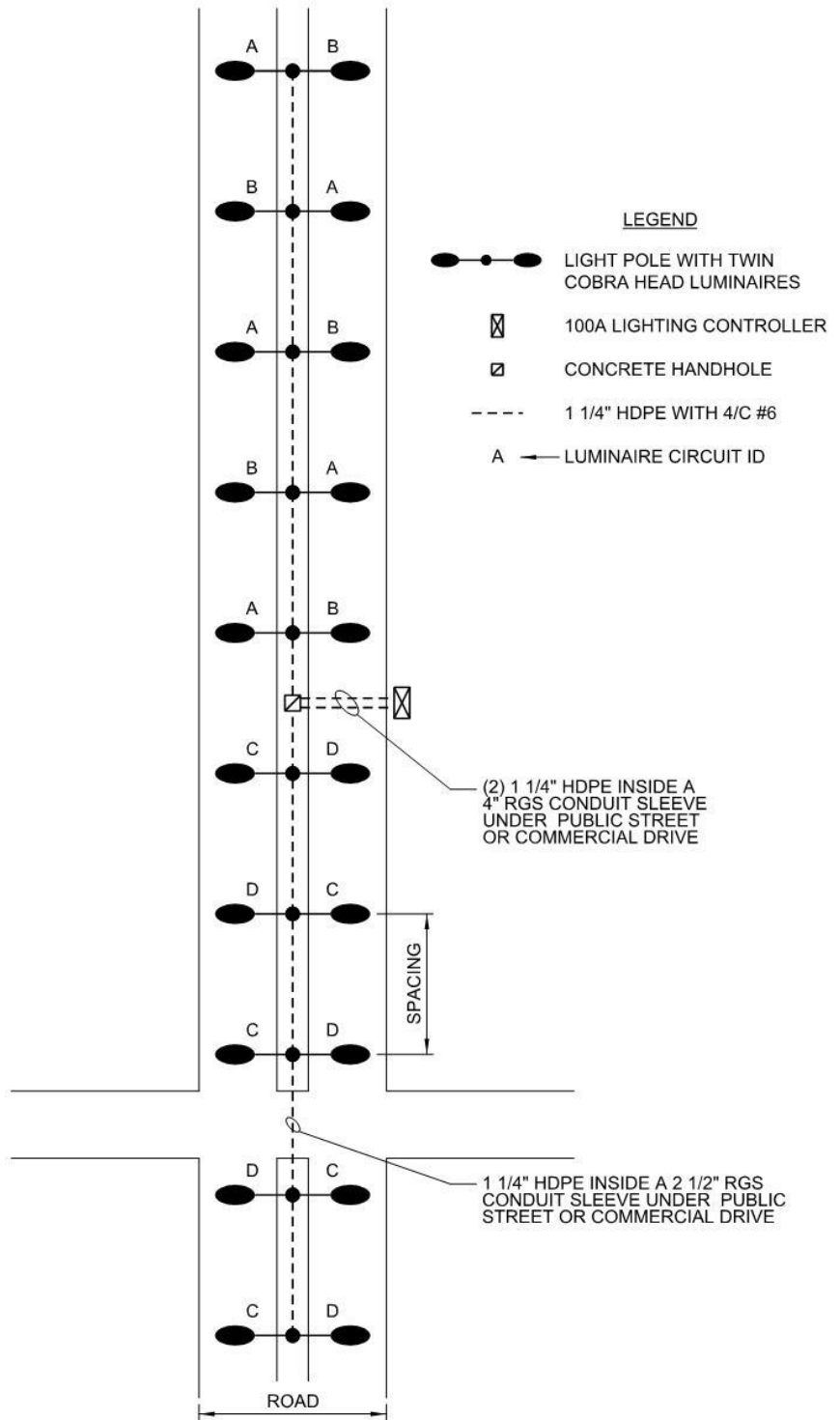
**CITY OF NAPERVILLE
TYPICAL STAGGERED POLE AND CIRCUIT LAYOUT WITH CONTROLLER**



CITY OF NAPERVILLE
TYPICAL OPPOSITE POLE AND CIRCUIT LAYOUT WITH CONTROLLER

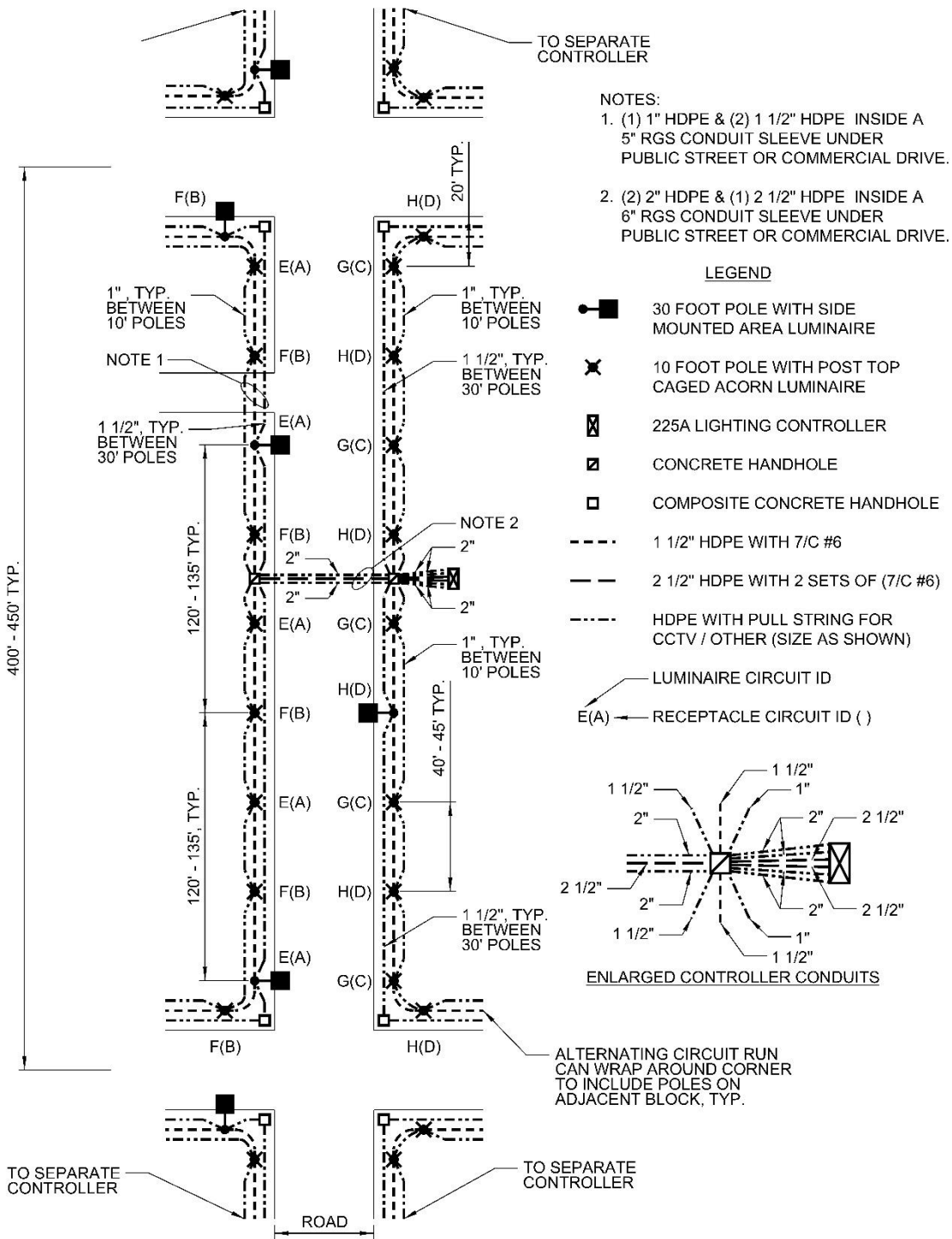


CITY OF NAPERVILLE
TYPICAL MEDIAN POLE AND CIRCUIT LAYOUT WITH CONTROLLER



CITY OF NAPERVILLE

TYPICAL CBD POLE AND CIRCUIT LAYOUT WITH CONTROLLER



SECTION 5 WATER DISTRIBUTION SYSTEM DESIGN

- 5.1 GENERAL DESIGN CRITERIA**
 - 5.1.1 Design Approval**
 - 5.1.2 IEPA Permit Required**
 - 5.1.3 Differentiation Between Public and Private Water Mains**
 - 5.1.4 Easements**

- 5.2 WATER MAINS**
 - 5.2.1 Basis of Design**
 - 5.2.2 Pipe Material**
 - 5.2.3 Location in the Public Right-Of-Way**
 - 5.2.4 Sewer and Water Main Separation**
 - 5.2.4.1 Horizontal Separation**
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 - 5.2.5 Depth of Pipe Cover**
 - 5.2.6 Water Main Sizing**
 - 5.2.7 Oversizing Requirements**
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 - 5.2.9 Gridiron or Looping**

- 5.3 WATER SERVICES**
 - 5.3.1 Pipe Material**
 - 5.3.2 Water Service Line Sizing**
 - 5.3.3 Water Service Upgrade/Replacements**
 - 5.3.4 Pipe Material**
 - 5.3.5 Water Service Line Sizing**

- 5.4 FIRE PROTECTION**
 - 5.4.1 Fire Protection Supplies**
 - 5.4.1.1 Public Fire Flow Requirements**
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 - 5.4.2 Fire Hydrant Locations**

- 5.5 VALVES**
 - 5.5.1 Valve Boxes and Valve Vaults**
 - 5.5.2 Valve Locations**
 - 5.5.3 Pressure Taps**

- 5.6 APPURTENANCES**
 - 5.6.1 Casing Pipes**
 - 5.6.1.1 Casing Pipe Material**
 - 5.6.1.2 Sizing of Casing Pipes**

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SECTION 5 WATER DISTRIBUTION SYSTEM DESIGN

5.1 GENERAL DESIGN CRITERIA

5.1.1 Design Approval

The Department of Public Utilities- W/WW shall approve all designs and shall alter the following design requirements as necessary to meet the City water system plan. The Department of Public Utilities and the Transportation Engineering and Development Business Group shall review and comment on all designs.

The City of Naperville's standard details, as currently adopted by the Department of Public Utilities- W/WW, should be used in all construction plans, both new development and single family residential, shall be in compliance. Any modifications to these standard specifications and details require approval from the Director of Public Utilities- W/WW.

5.1.2 IEPA Permit Required

All public water main extensions require an IEPA Construction permit, prior to construction.

5.1.3 Differentiation Between Public and Private Water Mains

When the water distribution system is complete and the improvements have been formally accepted by the City, the Department of Public Utilities- W/WW shall be responsible for the repairs and maintenance of all service lines and water mains located in the public right-of-way and publicly dedicated water mains and loop systems. Maintenance and repairs for the portion of the system located on private property shall be the responsibility of the property owner, or property owner's association or a property owner's association depending upon the property owner's covenants and guidelines. In such cases where the City performs maintenance or repairs on the private water distribution system, the City reserves the right to charge the property owner for necessary work.

All engineering plans shall clearly differentiate between all portions of the public and private water distribution system.

5.1.4 Easements

All public water mains and fire hydrants that are not located within a public right of way shall be placed in a public utility and drainage easement, minimum 15-feet wide or as directed by the Department of Public Utilities- W/WW, to the City of Naperville. The easement shall be granted to the City either through a recorded plat of subdivision or a recorded plat of easement. The City shall be granted access to these easements if not directly adjacent to public right-of-way.

At the discretion of the Director of Public Utilities- W/WW, the City may require additional easements for future maintenance or repair of the system, even those portions which may be located within the public right-of-way.

5.2 WATER MAINS

5.2.1 Basis of Design

All water systems shall be designed using the Hazen-Williams method. The coefficient of roughness shall equal one hundred ($c = 100$).

5.2.2 Pipe Material

Ductile iron pipe shall be cement-lined, have a minimum Class 52 thickness designation, and polyethylene encasement. For information regarding applicable ANSI, AWWA, and ASTM designations, refer to Section 502 of the City of Naperville Standard Specifications.

5.2.3 Location in the Public Right-Of-Way

Water mains shall generally be located on the north and west sides of the public right-of-way, or as directed by the Director of Public Utilities- W/WW.

5.2.4 Sewer and Water Main Separation

Water mains and water service lines shall be protected from sanitary sewers, storm sewers, combined sewers, house sewer service connections, drains, and sanitary sewer force main. In addition to the following guidelines, refer to Section 503 of the Standard Specifications.

5.2.4.1 Horizontal Separation

Water mains shall be laid at least 10 feet horizontally from any existing or proposed drain, storm sewer, sanitary sewer, combined sewer or sewer service connection.

Water mains may be laid closer than 10 feet to a sewer line when:

- Local conditions prevent a lateral separation of ten feet;
- The water main invert is at least 18 inches above the crown of the sewer; and
- The water main is either in a separate trench or in the same trench on an undisturbed earth shelf located to one side of the sewer.

When it is impossible to meet the conditions above, the drain or sewer shall be constructed of slip-on mechanical joint cast or ductile iron pipe, or PVC SDR 26 pipe meeting the requirements for water main. The drain or sewer shall be pressure tested to the maximum expected surcharge head before backfilling with no leakage allowed in the area of required water main protection. A City of Naperville representative shall witness this testing.

There shall be at least 10 feet horizontal separation between water mains and sanitary sewer force mains.

Water mains must be separated at least 25 feet from septic tanks, disposal fields, seepage beds, and sewage lift stations. Watermain must be separated from underground fuel storage tanks by a minimum of 25' and fluoroelastomer gaskets must be provided. 50 feet of separation shall be provided if practical.

5.2.4.2 Vertical Separation

A water main shall be laid so that its invert is 18 inches above the crown of the drain or sewer whenever water mains cross storm sewers or sewer service connections. The vertical separation shall be maintained for that portion of the water main located within 10 feet horizontally or any sewer or drain crossed. A length of water main pipe shall be centered over the sewer to be crossed with joints equidistant from the sewer or drain.

The sewer shall be constructed of slip-on or mechanical joint cast or ductile iron pipe, or PVC SDR 26 pipe, meeting the requirements for water main when:

- It is impossible to obtain the proper vertical separation as described above; or
- The water main passes under a sewer or drain.

A vertical separation of 18 inches between the invert of the sewer or drain and the crown of the water main shall be maintained where a water main crosses under a sewer. Support the sewer or drain lines to prevent settling and breaking of the water main.

If the invert of the water main is not 18 inches above the crown of the sewer when the pipes cross, a casing pipe can be installed around either the water main or sewer in lieu of constructing the sewer with water main equivalent pipe. The casing pipe must be a material that is approved for use as water main. The casing must extend on each side of the crossing at least 10 feet as measured at right angles from the outside edge of water main pipe to the outside edge of the sewer pipe. Pipe support shall be provided within the casing pipe and ends of the casing shall be filled with an approved non-shrink grout.

At crossings when the invert of the water main is not 18 inches above the crown of the storm sewer, and the sewer crossed the water main at right angles, the storm sewer can be constructed with reinforced concrete pipe using flexible gasket joints meeting ASTM C-361 or ASTM C-443 instead of providing a casing pipe or constructing the storm sewer with water main equivalent pipe. If gasketed storm sewer piping is proposed, it shall be installed between adjacent storm structures. The drain or sewer shall be pressure tested to the maximum expected surcharge head before backfilling with no leakage allowed in the area within 10 feet of the water main. A City of Naperville representative shall witness this testing.

Construction shall extend on each side of the crossing until the distance from the water main to the sewer or drain line is at least 10 feet as measured at right angles from the outside edge of the water main pipe to the outside edge of the sewer pipe.

Where a water main passes over an existing or proposed forcemain, an 18-inch vertical separation shall be provided at the crossing. (Note: A forcemain shall not be allowed to be above the water main at the crossing.)

5.2.5 Depth of Pipe Cover

All pipe shall be laid to a minimum depth of 5 feet measured from the proposed ground surface to the top of the pipe, unless specifically allowed otherwise in special circumstances by the Director of Public Utilities. If approved, the pipe shall be insulated with 2-inch exterior grade rigid insulation board. One 2-inch thick sheet of insulation shall be provided for each 6 inches of cover below the required 5-foot minimum depth and extend a minimum of 12 inches on each side of the water main. The insulation shall have a minimum R-value of R-9, and comply with ASTM C 578-92 Type 1X.

5.2.6 Water Main Sizing

Water mains shall be constructed of 8-inch diameter or 12-inch diameter pipe, or as directed by the Department of Public Utilities.

5.2.7 Oversizing Requirements

It may be determined that the water main be oversized in order to provide service to additional benefiting properties. Section 7-3-6 of the Municipal Code (City participation in construction of public improvements) outlines the City's policy relative to cost sharing for oversizing.

5.2.8 Limits of Installation

At a minimum, water main shall extend across the frontage of the property, at the developer's cost, such that a connection could be made with minimal disturbance in the future. In some cases, the City may require that the water main be installed from one corner to the diagonally opposite corner, at the developer's cost.

All water main stubs for future extension shall terminate with a valve and hydrant. Restrained joints shall be located 40 feet from the capped end.

5.2.9 Gridiron or Looping

Each 8-inch diameter water main shall be looped at a distance not to exceed 1,000 feet.

Each 12-inch diameter and larger water main shall be looped at a distance not to exceed 3,000 feet.

No dead-ended water mains shall be allowed.

5.3 WATER SERVICES

No more than one building shall be supplied from one service pipe. Whenever possible, the service pipe shall enter the building in a direct line with the curb stop and tap. Said pipe shall be provided with a valve before and after the water meter, not less than 1-foot inside of the wall or above the foundation floor (main building shutoff valve). The property owner shall be responsible for the maintenance, repair or replacement of the main shutoff valves.

Every building, except auxiliary buildings shall be individually metered.

All valves shall be arranged so that each line can be shut off from the exterior of the building. All water meters shall have approved ball valves installed on both sides of the meter.

5.3.1 Water Service Sizing

Water services shall be sized per Illinois Plumbing Code requirements based on the water supply fixture unit count in each dwelling. Minimum water service size shall be 1”.

5.3.2 New Water Service Installations

All water services shall be installed per Utility requirements and sizing shall remain consistent from the tap at the main to the meter,

5.3.3 Water Service Upgrades/Replacements

Water service upgrades/replacements shall be installed per Utility requirements; the existing un-used water service is required to be terminated at the main.

Every building, except auxiliary buildings shall be individually metered.

Where applicable, domestic water services shall be tapped from fire lines inside the building per Illinois Department of Public Health requirements to prevent dead end piping and maintain water quality. Where new fire lines are installed in existing buildings, the existing domestic water service shall be tapped from the new fire line to meet Illinois Department of Public Health requirements. The existing water service shall be terminated at the main as required. A sprinkler room with separate exterior access shall be provided for the fire line and fire alarm equipment

For attached Single Family Buildings (Townhomes/Rowhouses, 3 units or more), the following options shall apply.

5.3.3.1 Option 1

Designated fire sprinkler line shall be installed for the entire building, this fire line will enter into a separate riser room with outside access. One (1) City revenue water meter will be tapped from this fire line inside the riser room and will meter water usage for all units with 1 water bill to house meter account. Both fire line and domestic water lines to be sized by the design professional. In this case there will be one (1) sanitary service for the entire building.

5.3.3.2 Option 2

Designated fire sprinkler line shall be installed for the entire building, this fire line will enter into a separate riser room with outside access. The unit adjacent to the riser room will tap their domestic water service from the fire line inside the riser room, this adjacent unit's water meter will be installed inside the riser room. All remaining units will tap domestic water service from the public main and install b-boxes in the ROW, water meters for these units will be installed inside the units. All units will be billed separately. Both fire line and domestic water lines to be sized by the design professional. In this case, separate sanitary services will be required for each unit.

5.3.3.3 Option 3

All units shall install individual and properly sized domestic water services to provide domestic water and fire protection to each unit separately. B-boxes for each unit installed in the ROW, water meters installed inside each unit. All units will be billed separately. Water service sized by the design professional. In this case, separate sanitary services will be required for each unit.

- DPU-W/WW IAC fees and meter charges will apply for each water service and meter that is installed.

5.3.4 Pipe Material

Service lines 2 inches or less (≤ 2 "), inside diameter, shall be copper pipe, "Type K". All services 3 inches or greater (≥ 3 "), inside diameter, shall be Class 52 ductile iron pipe with cement lining.

5.3.5 Water Service Line Sizing

The following guidelines shall be used to size water service lines:

Water Supply Fixture Units	Size, diameter (inches)
< 60	1"
≥ 60	1-1/2"

5.4 FIRE PROTECTION

5.4.1 Fire Protection Supplies

Fire flow requirements will be approved by the Director of Public Utilities.

The required fire flows may be computed at a residual pressure of 20 psi for Fire Department use. It should be recognized that higher residual pressures may be necessary for specific fire protection demands.

The following flow requirements are guidelines which will be modified based upon construction types, size and height of the buildings:

Use	Flow (gpm)
Manufacturing and Storage	3,000 to 5,000
Institutional (Assumed limited hazard, fire resistive construction, or automatic protective devices provided)	3,000 to 4,000
Commercial and Mercantile	3,000 to 5,000
Business and Office (Assumed limited hazard)	2,500 to 3,500
Single Family Detached Residential (1,000 gpm typically used for average home size and spacing)	1,000 to 1,500
Town or Row Houses (1,500 gpm average, additional quantity due to risk of fire to adjoining structures or units)	1,000 to 2,000
Apartments (Low-end requirements for fire resistive structures, higher-end requirements for wood frame, similar construction)	3,000 to 4,000

5.4.1.1 Public Fire Flow Requirements

Public fire flow requirements (Fire Department pumper supply) and domestic consumption demands are cumulative.

5.4.1.2 Private Fire Protection

Private fire protection demands (standpipe systems, sprinkler systems, etc.) will not be considered cumulative with public fire flow demands. These requirements are based upon the building height, area, use and construction type.

5.4.2 Fire Hydrant Locations

Fire hydrants shall be located 7.5 feet from the right-of-way within north or west parkways.

All points of buildings shall be within 300 feet of all points of the building, as the hose lays (must be shown on engineering plans).

The linear spacing of hydrants shall not exceed 350 feet in single-family areas or 300 feet elsewhere.

Fire hydrants shall be situated in the parkway within 10 feet from a paved roadway and 50 feet from a Fire Department connection (Siamese).

Access to hydrants shall be provided by paved roadways (minimum structural number = 2.5) routes adequate in width, clearance and strength to support all fire equipment. Such routes shall be maintained during all seasons of the year.

5.5 VALVES

5.5.1 Valve Boxes and Valve Vaults

Valve boxes may only be used for fire lines that are less than 8 inches in size. The valve box shall be located in a grassy area; otherwise, a valve vault must be used.

Valves for water mains or fire lines 8 inches or larger shall be provided with a vault.

5.5.2 Valve Locations

Valves shall be located such that no more than a maximum of 500 feet to 700 feet of main may be shut off at any given time. This spacing should be reduced, such that no more than 25 to 30 single-family homes, or 50 multi-family residences (excluding apartment buildings) would be shut off at any given time.

Where a “tee” is installed, at least two (2) valves will be utilized.

Where a “cross” is installed, at least three (3) valves will be utilized.

5.5.3 Pressure Taps

Pressure taps shall be cast iron or ductile iron tapping sleeves for size-on-size taps, or taps on mains greater than 12 inches (> 12”). Stainless steel tapping sleeves can only be used for non-size-on-size taps or taps on mains less than or equal to 12 inches (≤ 12”).

5.6 APPURTENANCES

5.6.1 Casing Pipes

Manufactured non-metallic or non-corrosive casing spacers, adjustable runners, or cradles shall be used to support the pipe in the casing. A minimum of three supports shall be used per pipe, or per manufacturer’s recommendation. The annular space shall be filled with pea gravel or as required by the permitting agency, and provisions shall be made so that no voids are left. Water main installed within casing pipes shall have restrained joint construction the entire length of the casing pipe for future removal if necessary.

Where practical, alternative pipe materials may be used in lieu of casing pipe.

5.6.1.1 Casing Pipe Material

The steel casing pipe shall be bituminous coated, minimum of 30 mils thickness inside and out, and shall be of leak proof construction, capable of withstanding the anticipated loadings. The steel casing pipe shall have a minimum yield strength of 35,000 psi and shall meet the requirements of ASTM A139, Grade B. Ring deflection shall not exceed 2% of the nominal diameter. The steel casing pipe shall be delivered to the jobsite with beveled ends to facilitate field welding.

<u>Steel Casing Diameter</u>	<u>Minimum Wall Thickness (Inches)</u>
20” and 22”	0.344
24”	0.375
28”	0.438
30”	0.469
32”	0.501
34” and 36”	0.532

5.6.1.2 Sizing of Casing Pipes

The diameter of the casing pipe shall be a minimum of 12 inches greater than the outside nominal diameter of the water main.

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